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PRESIDENCY UNIVERSITY

BENGALURU

End - Term Examinations - MAY 2025

School: SOE	Program: Civil Engineering			
Course Code: CIV3047	Course Name: Fundamentals of PreStressed Concrete Design			
Semester: VI	Max Marks: 100	Weightage: 50%		

CO - Levels	CO1	CO2	СО3
Marks	28	48	24

Instructions:

- (i) Read all questions carefully and answer accordingly.
- (ii) Do not write anything on the question paper other than roll number.

Part A

Answer ALL the Questions. Each question carries 2marks.

10Q x 2M=20M

1.	Define the following terms (i) Partial prestressing (ii) Full prestressing.	2 Marks	L1	CO1
2.	List the benefits of pre stressed concrete structures.	2 Marks	L1	CO1
3.	Outline the differences between RCC and PSC structures.	2 Marks	L1	CO1
4.	Explain hydrogen embrittlement in steel.	2 Marks	L2	CO1
5.	List the various types of loss of pre stress in pre tensioned concrete.	2 Marks	L1	CO2
6.	With usual notations, write the expression used to calculate loss of pre stress due to elastic deformation of concrete.	2 Marks	L1	CO2
7.	What are anchorage slip and friction losses in PSC members?	2 Marks	L1	CO2
8.	Outline the factors influencing deflections in PSC sections.	2 Marks	L1	CO2
9.	List the assumptions made in strain compatibility method of analysis of beams.	2 Marks	L1	CO3
10.	Describe briefly the flexural failure of over reinforced sections.	2 Marks	L2	CO3

Part B

Answer the Questions.

Total Marks 80M

11.		A rectangular concrete beam 200 mm wide and 000 mm dage	10 Marks	L3	CO1
11.	a.	A rectangular concrete beam 300 mm wide and 800 mm deep	TO MALKS	L3	COI
		supports two concentrated loads of 20 kN each at the third point			
		of a span of 9m:			
		a) Suggest a suitable cable profile. If the eccentricity of the cable			
		profile is 100 mm for the middle third portion of the beam,			
		calculate the prestressing force required to balance the bending			
		effect of the concentrated loads (neglect self-weight of beam)			
		b) For the same cable profile, find the effective force in the cable			
		if the resultant stress due to all the loads is zero at the bottom			
		fibre of the mid-span section.			
	b.	The cross-section of a prestressed concrete beam used over a	10 Marks	L3	CO1
		span of 6m is 100 mm wide and 300 mm deep. The initial stress			
		in the tendons located at a constant eccentricity of 50 mm is			
		1000 N/mm ² . The sectional area of the tendons is 100 mm ² .			
		Determine the percentage increase in stress in the wires when			
		the beam supports a live load of 4 kN/m. Density of concrete is			
		24 kN/m ³ . Modulus of elasticity of concrete is 36 kN/mm ² and			
		steel is 210 kN/mm ² respectively.			
		Or			
12.	a.	A rectangular prestressed beam 150 mm wide and 300 mm deep	10 Marks	L3	CO1
12.	a.	is used over an effective span of 10m. The cable with zero	10 Marks	ЦЭ	COI
		eccentricity at the supports and linearly varying to 50 mm at the			
		centre, carries an effective prestressing force of 500 kN. Find the			
		magnitude of the concentrated load Q located at the centre of the			
		span for the following conditions:			
		• If the load counteracts the bending effect of the prestressing			
		force (neglect self-weight of beam)			
		• If the pressure line passes through the upper kern of the			
		section under the action of external load, self-weight, and			
		prestress.			
	b.	A rectangular concrete beam of cross section 120 mm wide and	10 Marks	L3	CO1
		300 mm deep is prestressed by a straight cable carrying an			
		effective force of 180 kN at an eccentricity of 50 mm. The beam			
		supports an imposed load of 3.14 kN/m over a span of 6m. If the			
		modulus of rupture of concrete is 5 N/mm², calculate the load			
		factor against cracking assuming the density of concrete as 24			
		kN/m³.			
13.	a.	A pre-tensioned beam having 200mm width and 300 mm depth	20 Marks	L3	CO2
		is prestressed by 10 wires of 7mm diameter initially stressed to			
Ī		1200N/mm ² , with their centroids located 100mm from the			

tensioned wires, each of 2mm diameter, uniformly distributed over the section. The wires are initially tensioned on the prestressing bed with a total force of 300 kN. Calculate the final stress in concrete and the percentage loss of stress in steel after all losses. Assume E ₆ = 210 kN/mm² and E ₆ = 32 kN/mm². Creep coefficient = 1.5, Shrinkage strain = 200x10·6, Relaxation of steel stress = 5% of initial stress. 15. a. A prestressed concrete beam of rectangular section 120mm wide x 300mm deep, spans over 6m. The beam is prestressed by a straight cable carrying effective force of 200kN at an eccentricity of 50mm. Take Ec as 38kN/mm² and p ₆ = 24 kN/m³. Determine the deflection at center of span under prestress and self-weight. Also calculate the magnitude of the uniformly distributed live load which will nullify the deflection due to prestress and self-weight. b. A prestressed concrete slab bridge deck designed for IRC loads has the following parameters. Estimate the long-term deflection of the slab using the if the effective span of the slab is 10.4m. Take the total dead load as 14 kN/m, live load at center of span as 93.2 kN, and effective prestressing force after losses is 1350 kN/mm width of slab. Assume E _c = 33 kN/mm², I = 10.4x10° mm², Eccentricity of parabolic cable at center of span is 195 mm and creep coefficient as 2.5. Or 16. a. The deck of prestressed concrete culvert is made up of a slab of 500 mm thick. The slab is spanning over 10.4 m and supports at total UDL of 33.5 kN/m, including DL and LL. The modulus of elasticity of concrete is 38 kN/mm². The concrete slab is prestressed by a straight cable each containing 12 high tensile wires of 7mm diameter stressed to 1200 N/mm² at a constant eccentricity of 195 mm. The cables are spaced at 328 mm intervals in the transverse direction. Estimate the instantaneous deflection of the slab at center of span under prestress and the imposed loads.						
a. A prestressed concrete pile, 250x250 mm, contains 60 pretensioned wires, each of 2mm diameter, uniformly distributed over the section. The wires are initially tensioned on the prestressing bed with a total force of 300 kN. Calculate the final stress in concrete and the percentage loss of stress in steel after all losses. Assume E _s = 210 kN/mm² and E _c = 32 kN/mm², Creep coefficient = 1.5, Shrinkage strain = 200x10-6, Relaxation of steel stress = 5% of initial stress. 15. a. A prestressed concrete beam of rectangular section 120mm wide x 300mm deep, spans over 6m. The beam is prestressed by a straight cable carrying effective force of 200kN at an eccentricity of 50mm. Take Ec as 38kN/mm² and ρ _c = 24 kN/m³. Determine the deflection at center of span under prestress and self-weight. Also calculate the magnitude of the uniformly distributed live load which will nullify the deflection due to prestress and self-weight. b. A prestressed concrete slab bridge deck designed for IRC loads has the following parameters. Estimate the long-term deflection of the slab using the if the effective span of the slab is 10.4m. Take the total dead load as 14 kN/m, live load at center of span as 93.2 kN, and effective prestressing force after losses is 1350 kN/mm width of slab. Assume E _c = 33 kN/mm², I = 10.4x10° mm⁴, Eccentricity of parabolic cable at center of span is 195 mm and creep coefficient as 2.5. 10 The deck of prestressed concrete culvert is made up of a slab of 500 mm thick. The slab is spanning over 10.4 m and supports a total UDL of 33.5 kN/m, including DL and LL. The modulus of elasticity of concrete is 38 kN/mm². The concrete slab is prestressed by a straight cable each containing 12 high tensile wires of 7mm diameter stressed to 1200 N/mm² at a constant eccentricity of 195 mm. The cables are spaced at 328 mm intervals in the transverse direction. Estimate the instantaneous deflection of the slab at center of span under prestress and the imposed loads. b. A post-tensioned roof girder spanning over 30m h			after transfer, allowing only for elastic shortening of concrete. If the concrete further undergoes shortening due to creep and shrinkage while there is relaxation of 5% of steel stress, calculate the final percentage loss of stress. Take $E_s=210 kN/mm^2$, $f_{ck}=42N/mm^2$, Creep Coefficient, $\Phi=1.6$, Total residual shrinkage strain = $3x10^{-4}$.			
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unsymmetrical I section with second moment of area of section		b.		10 Marks	L3	CO2
			unsymmetrical I section with second moment of area of section			

as 72490 x 10 ⁶ mm ⁴ and an overall depth of 1300 mm. The	
effective eccentricity of the group of cables is 580 mm at the	
centre of span and 170 mm at the supports. The cables carry an	
initial prestressing force of 3200 kN. The self-weight of the	
girder is 10.8 kN/m and the live load on the girder is 9 kN/m.	
The modulus of elasticity of concrete is 34 kN/mm ² . If the creep	
coefficient is 1.6, and the total loss of prestress is 15%, calculate	
the deflections at the following stages and compare them with	
the permissible values according to the Indian Standard Code	
(IS:1343) limits:	
(i) Instantaneous deflection due to prestress and self-weight	
(ii) Resultant maximum long-term deflection allowing for	
loss of prestress and creep of concrete.	

17.	a.	A pretensioned prestressed concrete beam having a rectangular section with a width of 150 mm and overall depth of 350 mm is prestressed by tendons of effective area 461 mm ² at an effective depth of 300 mm. Assuming a characteristic strength of concrete and steel as 40 and 1600 N/mm ² , estimate the ultimate flexural strength of the section using the provisions of IS 1343 code.	15 Marks	L3	CO3
	b.	Describe the assumptions made in strain compatibility method of flexural analysis of PSC members.	5 Marks	L2	CO3
		0r			
18.	a.	A pretensioned prestressed concrete beam having a rectangular section with a width of 200 mm and overall depth of 400 mm is prestressed by tendons of effective area 450 mm² at an effective depth of 350 mm. Assuming a characteristic strength of concrete and steel as 40 and 1500 N/mm², estimate the ultimate flexural strength of the section using the provisions of IS 1343 code.	15 Marks	L3	CO3
	b.	Explain the various types of flexural failure of PSC beams.	5 Marks	L2	CO3