



ROLL NO. \_\_\_\_\_

**PRESIDENCY UNIVERSITY, BENGALURU**  
**SCHOOL OF ENGINEERING**

Max Marks: 40

Max Time: 120 Mins

Weightage: 40 %

**END TERM FINAL EXAMINATION**

I Semester AY 2017-2018

Course: **CIV 214 Foundation Engineering**

20 DEC 2017

**Instructions:**

- i. Write legibly
- ii. Scientific and non-programmable calculators are permitted.

**Part A**

(2Q x 4 M= 8Marks)

1. What are the different components of settlements for shallow footing? What is meant by flexible and rigid footing?
2. Classify the Pile foundations on different basis (in a tabular column).

**Part B**

(4 Q x 4 M= 16 Marks)

3. A rectangular footing 3m x 1.5m exerts a pressure of 80KN/m<sup>2</sup> on a cohesive soil ( $E_s = 5.1 \times 10^4$  KN/m<sup>2</sup>,  $\mu = 0.50$ ). Determine the immediate settlement at the center, assuming the footing is (a) flexible and (b) rigid. Flexible influence factors for  $L/B = 2$  are 1.53 at the center and 0.77 at the corner and for rigid it is 0.8.
4. A concrete Pile 50cm in diameter and 20m long is driven into a homogeneous mass of clay soil of medium consistency. The water table is at the ground surface. The unit cohesion is 45 KN/m<sup>2</sup> and adhesion factor  $\alpha = 0.70$ . Compute  $Q_u$  and  $Q_a$  with FS 2.5.
5. A square pile group of 16 piles, 12m long, passed through a filled up soil of 4m depth. The pile diameter is 300mm and the pile spacing is 800mm. If unit cohesion of the material is 18 KN /m<sup>2</sup> and the unit weight is 15KN/m<sup>3</sup> compute the negative skin friction of the group.
6. A pile of 0.45m diameter is driven in saturated loose sand as shown in the figure 1. Estimate the ultimate load carrying capacity from static formula.

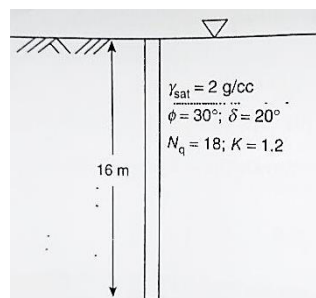


Figure 1

**Part C**

(2 Q x 8M= 16 Marks)

7. Calculate the total consolidation settlement of soil profile as shown in figure 2, composed of two different types of clay, ie clay 1 and clay 2 due to 150 kpa net foundation loading. Take unit weight of water as 10 KN/m<sup>3</sup> and assume that Skempton-Bjerrum correction factor  $\mu = 0.7$  for both clay layers. Note that  $P_c$  is the preconsolidation pressure.

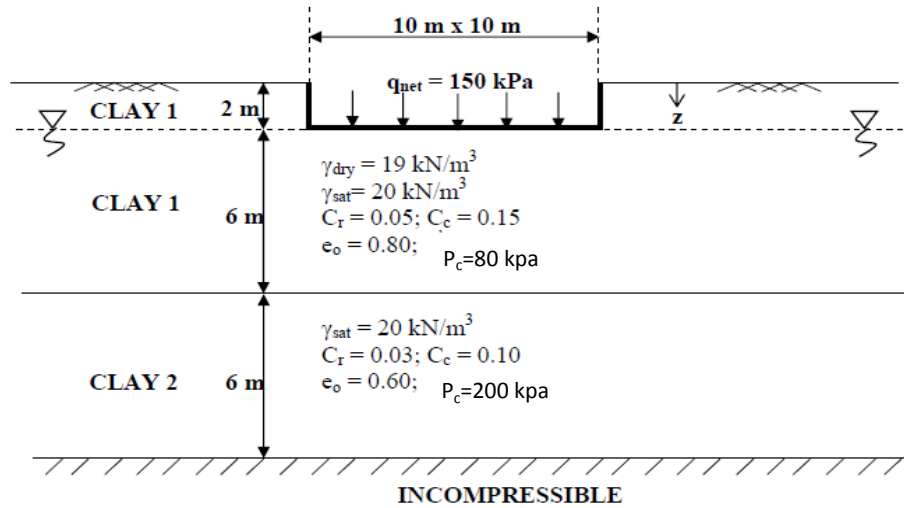


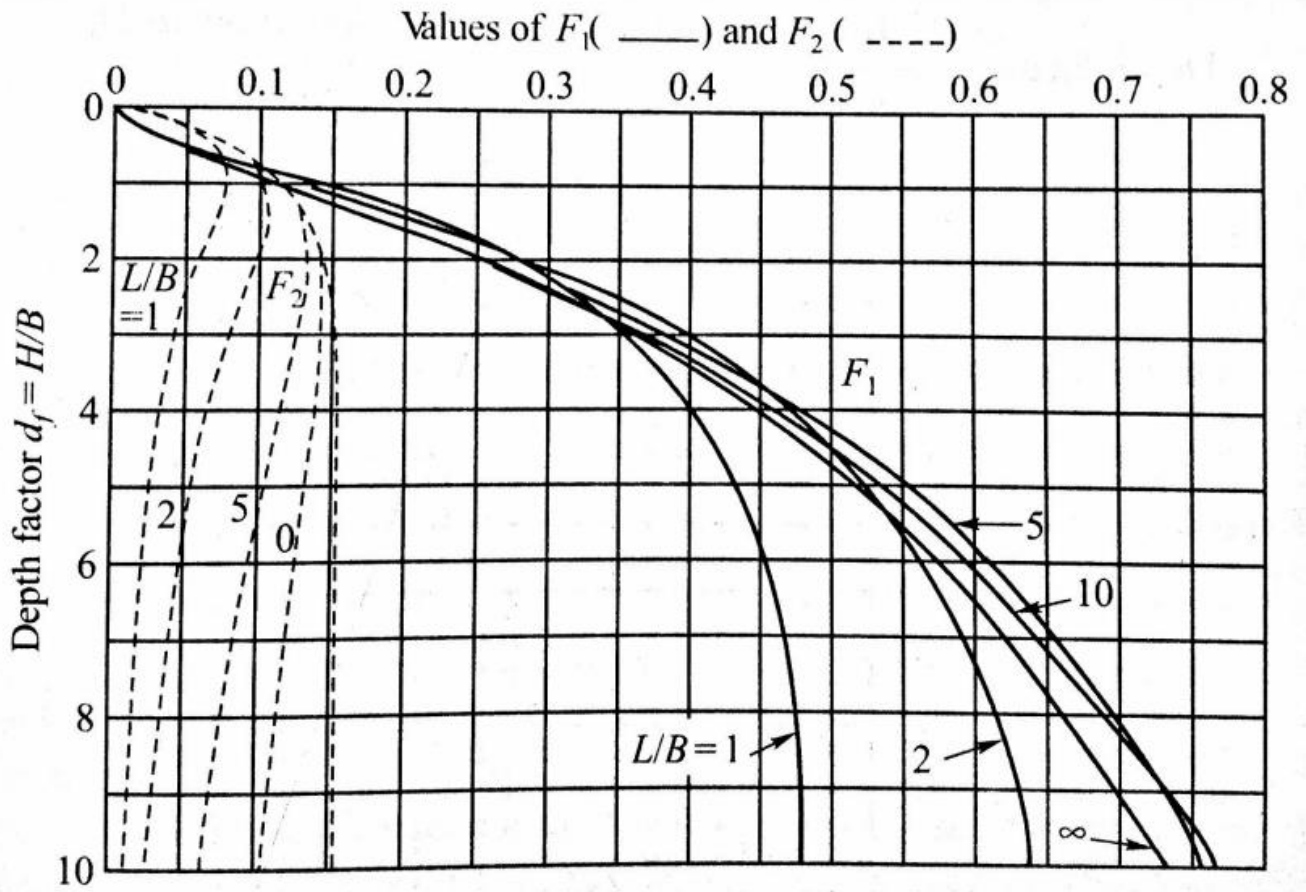
Figure 2

8. A square footing of size 10m x 10m is founded at a depth of 2m below the ground surface in loose medium dense sand with  $q_n = 120 \text{ KN/m}^2$ . Standard penetration tests conducted at the site gave the following  $N_{60}$  values:

Depth below GL in m	$N_{60}$	Depth below GL in m	$N_{60}$
2	8	12	16
4	8	14	18
6	12	16	17
8	12	18	20
10	11		

The water table is at the base of the foundation. Above the water table,  $\gamma = 16.5 \text{ KN/m}^3$  and submerged  $\gamma_b = 8.5 \text{ KN/m}^3$ . Compute the elastic settlement by using,  $E_s = 250(N_{\text{corr}} + 15)$  for computing the modulus of elasticity of the sand. Assume  $\mu = 0.3$  and depth of compressible layer =  $2B = 20\text{m}$  ( $=H$ )

REFERENCE FIGURE FOR QUESTION 8





# PRESIDENCY UNIVERSITY, BENGALURU

## SCHOOL OF ENGINEERING

Max Marks: 20

Max Time: 60 Mins

Weightage: 20 %

### TEST 2

I Semester 2017-2018

Course: CIV 214 Foundation Engineering

25 OCT 2017

#### Instructions:

- i. Write legibly
- ii. Scientific and non-programmable calculators are permitted.

#### Part A

(2Q x 3 M= 6 Marks)

1. What are the different modes of failure of finite slopes? Explain with diagram.
2. What are the different types of bearing capacity failure? Explain with a diagram.

#### Part B

(2 Q x 4 M= 8 Marks)

3. A laboratory test gave the following soil parameters  $C = 35 \text{ KN/m}^2$ ,  $\phi = 32^\circ$ . The expected parameters of the mobilized shearing resistance are  $C_m = 20 \text{ KN/m}^2$ ,  $\phi_m = 25^\circ$ . The average effective pressure on the failure plane is  $165 \text{ KN/m}^2$ . Find the factors of safety w.r.t average shearing strength, cohesion and internal friction
4. A square footing  $3\text{m} \times 3\text{m}$  is built on a homogeneous bed of sand of density  $19 \text{ KN/m}^3$  and having  $\phi = 35^\circ$ . The depth of foundation is  $1.5\text{m}$  below the ground. Determine the safe load that can be applied on the footing with F.O.S of 3.0 considering  $N_c = 57.8$ ,  $N_q = 41.4$  and  $N_\gamma = 42.4$  as the bearing capacity factors for analysis.

#### Part C

(1 Q x 6 M= 6 Marks)

5. An embankment is to be constructed with  $C = 25 \text{ KN/m}^2$ ,  $\phi = 25^\circ$ ,  $\gamma = 19 \text{ KN/m}^3$ ,  $F_c = 1.3$ , height  $H = 12\text{m}$ . Estimate the side slope required. Taylor's stability numbers are as follows for slope angle.

Slope Angle	90	75	60	45	30	20	10
$S_n$	0.182	0.134	0.097	0.062	0.025	0.005	0

Also find the factor of safety if the slope angle is 1 V: 2 H, given  $\phi = 25^\circ$



# PRESIDENCY UNIVERSITY, BENGALURU

## SCHOOL OF ENGINEERING

Max Marks: 40

Max Time: 60 Mins

Weightage: 20 %

### TEST 1

I Semester 2017-2018

Course: **CIV 214 Foundation Engineering**

20 SEPT 2017

#### Instructions:

- i. Write legibly
- ii. Scientific and non-programmable calculators are permitted.

#### Part A

(4Q x 5 M= 20 Marks)

1. How is standard penetration test conducted in the field? Explain with a neat diagram
2. How is the depth and lateral extent of soil exploration determined?
3. With a neat sketch of soil sampler, define (i) Area ratio (ii) Recovery ratio (iii) Inside clearance (iv) Outside Clearance.
4. Compare Boussinesq's and Westergaard's theory.

#### Part B

(2 Q x 7 M= 14 Marks)

5. Calculate the stress by point load method of Boussinesq at a depth of 20m below the centre of a raft of size 30 x 12 m founded at a depth of 2.5m subjected to a uniform pressure of 150KN/m<sup>2</sup>. Neglect the effect of founding depth. Assume coordinate of the centre of the raft is (0,0)
6. What is meant by SPT 'N' value?  
A single concentrated load of 1000 KN acts at the ground surface. Construct an isobar for  $\sigma_z = 40$  kN/m<sup>2</sup> by making use of the Boussinesq equation.

#### Part C

(1 Q x 6 M= 6 Marks)

7. ABCD is a raft foundation of a multi-storey building as shown in Figure 1

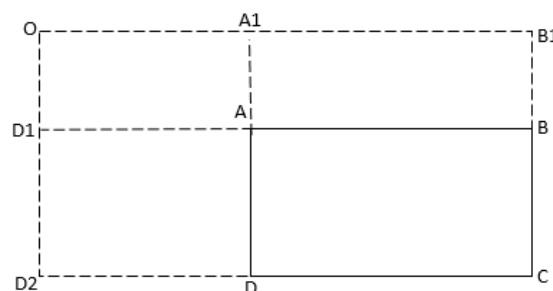


Figure 1

Where  $AB = 20\text{ m}$ ,  $BC = 12\text{ m}$ , the udl  $q$  over raft is  $350\text{ kN/m}^2$ . Determine  $\sigma_z$  at a depth of  $6\text{ m}$  below point  $O$  where  $AA_1 = 4\text{ m}$ ,  $A_1O = 6\text{ m}$ .

**FADUM'S CHART**

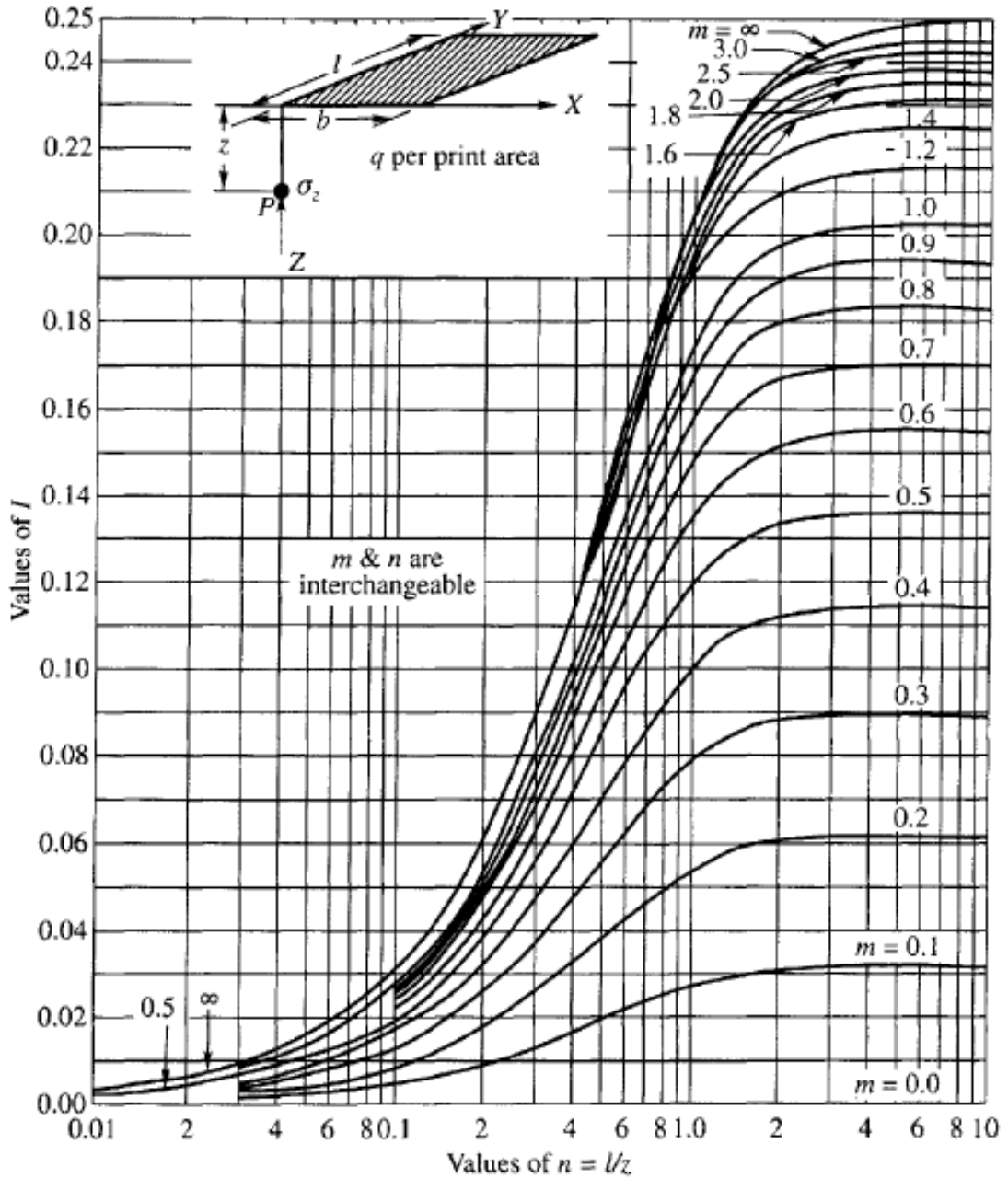


Figure 2